National Bank & Trust

Prepared for:
SLI Group, Inc.

Houston, Texas

Project No: 26-1119-0030
August 11, 2026

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August 11, 2026

SLI Group, Inc.
15810 Park Ten Place, Suite 300
Houston, TX 77084

Attention: Mr. Rickey Gates

Subject: Report for Geotechnical Consulting Services
National Bank & Trust
Sam Houston Drive and FM 969
Bastrop, Texas
Thompson Project No.: 26-1119-0030

Dear Mr. Gates:

Thompson Engineering (Thompson) is pleased to transmit this geotechnical report for the above-referenced project. Authorization for our services was made via email dated July 7, 2026. This document constitutes the geotechnical report, which may be used in support of project design and construction activities.

We appreciate the opportunity to assist you with project-related geotechnical matters. Please do not hesitate to contact our office with any questions concerning this submittal.

Respectfully submitted,
Thompson Engineering

Belchor Sebastiao, P.E.
Geotechnical Project Engineer

STATE OF TEXAS
FRANK S. ONG
83982
LICENSED
PROFESSIONAL ENGINEER
8/11/2026

Frank S. Ong, P.E.
Senior Geotechnical Engineer

Alabama | Florida | Georgia | Louisiana | Mississippi | North Carolina | Tennessee | Texas

10665 Richmond Avenue. Ste. 192 Houston, TX 77042
thompsonengineering.com | 713.972.6850 | 832.260.5342
Texas Board of Professional Engineers and Land Surveyors Firm Registration No. F-4824

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1.0 EXECUTIVE SUMMARY

Thompson has completed the geotechnical engineering services for the planned National Bank & Trust to be located at Sam Houston Drive and FM 969, Bastrop, Texas, in general accordance with the contracted scope of work between SLI Group, Inc. and Thompson. The geotechnical exploration program summarized in this document was performed following generally accepted local soil and geotechnical engineering practices.

This Executive Summary is provided as a brief overview of our geotechnical engineering evaluation and recommendations and is not intended to replace information detailed in the report.

- A total of four borings ranging from 5 to 25 feet were performed for this investigation to provide general subsurface data across the site.
- The general subsurface conditions revealed the presence of Silty Sand (SM) and Clayey Sand (SC) to the maximum explored depth of 25 feet.
- Groundwater was not encountered in our soil borings during our field drilling.
- The building can be supported by shallow spread footing foundations bearing at 2.5 ft below the lowest adjacent finished grade. Square foundations may be sized using a net allowable bearing pressure of 2.0 kips/ft^2 for total load. Strip foundations may be sized using a net allowable bearing pressure of 1.5 kips/ft^2 for total load. These bearing pressures may be increased by 33% for transient loads such as wind.

2.0 INTRODUCTION AND OBJECTIVES

The geotechnical exploration program summarized in this document was performed following generally accepted local soil and geotechnical engineering practices. This report presents the results of the geotechnical field exploration program, provides relevant technical discussions, and provides recommendations for geotechnical site preparation and foundation design/construction of proposed structures.

Thompson was commissioned for the engineering evaluation of geotechnical matters associated with the proposed site area. To satisfy the stated objective, the following tasks were undertaken:

- Explore the prevailing subsurface soil conditions in the general area of the proposed development.
- Utilize the collected subsurface data to classify and stratify the prevailing site soils and assign relevant soil strength and engineering properties.
- Evaluate the subsurface soil response to anticipated foundation loadings.
- Present a discussion of geotechnical matters deemed relevant and those with potential impact on the proposed engineering design and construction, as well as provide engineering conclusions and recommendations.

3.0 SCOPE OF SERVICES

The scope of geotechnical services deemed appropriate for this study, in light of the project requirements, our understanding of the project as summarized herein, and with reliance on our knowledge of the local geology, our experience at the vicinity of this project site, and with similar project parameters, included the following:

- A total of four borings to depths ranging from 5 to 25 feet below existing site grades were performed for this investigation to provide general subsurface data across the site.
- Interpretation of field data to determine site soil conditions and assessment of groundwater conditions.
- Laboratory testing to confirm soil classification.
- Engineering evaluations and recommendations for support of the design and construction of the planned structures.

4.0 PROJECT DESCRIPTION

We understand that the proposed project entails the design and construction of a 2,912 sq. ft. bank with a 3-lane drive-thru canopy and one lane ATM canopy. The new bank is expected to be wood framed building. Additionally, we assume that the finished grade of the building pad is within 2 feet of existing grade.

5.0 OBSERVED SITE CONDITIONS

The project site is generally covered with grass. Surface soil at boring locations and along the routes taken to access the boring locations were generally firm.

6.0 SUBSURFACE EXPLORATION PROGRAM

Subsurface drilling equipment was utilized to attempt to advance the four soil borings to depths ranging from 5 to 25 feet below the existing grade, using solid stem auger drilling techniques. The boreholes were backfilled upon fieldwork completion. The boring locations are presented in the "Boring Location Plan" attached in **Appendix A**.

The overall intent of the subsurface exploration program was to establish a site-specific subsurface conditions database. Within the boreholes, SPT sampling in cohesionless soils was performed using 2-inch O.D., 1.375-inch I.D. split-spoon samplers in general accordance with ASTM D1586 guidelines. The SPT total number of blows required to drive the sampler the second and third 6-inch increments is designated as the N-value and provides an indication of in-place soil strength, relative density and consistency.

Continuous sampling was performed within the upper 10 feet of the subsurface profile and at five (5) feet intervals thereafter to the termination depth of the soil borings at about 10 to 20 feet below existing grade.

7.0 LABORATORY SOIL TESTING PROGRAM

The soil samples were visually classified in general accordance with the guidelines of ASTM D2487, Standard Classification of Soils for Engineering Purposes (Unified Soil Classification System), and ASTM D2488, Standard Practice for Description and Identification of Soils (Visual Manual Procedure). Laboratory testing was conducted in accordance with ASTM D2216 (Moisture Content) and ASTM D1140 (Percent Finer than No. 200 sieve). The quantity and type of laboratory tests performed for this geotechnical study were determined and adjusted by Thompson's engineering personnel based on the uniformity and character of the subsurface soil conditions encountered and our experience and knowledge of local soil conditions. The results of these tests are documented in the respective columns of the Record of Test Borings, provided in Appendix A.

8.0 SUBSURFACE FINDINGS

While the establishment of generalized subsurface conditions is useful for the geotechnical engineering evaluation process, such generalizations should be reviewed with caution as they promote extrapolation of recovered data between soil test boring locations. The subsurface findings for this project are summarized below. See Appendix A for additional soil visuals.

8.1 Site Geologic Setting

Regionally, the project site is underlain by Fluviatile Terrace Deposits (Quaternary Period; Pleistocene Epoch). These deposits are described as stream terrace sediments consisting of three or more terrace levels that may correspond to coastal Pleistocene units. The deposits generally comprise varying proportions of gravel, sand, silt, and clay, with gravel becoming more prominent in the older and higher terraces. Along the Colorado River, the gravels are predominantly composed of quartz, while along the Guadalupe River they are largely siliceous and coarse, containing dolomite, limestone, chert, quartz, and various igneous and metamorphic rock fragments derived from the Llano region and Edwards Plateau. (U.S. Geological Survey)

8.3 Subsurface Conditions

Our assessment of the subsurface conditions is based on established correlations of the sampling data to soil type behavior. The general subsurface conditions revealed the presence of Silty Sand (SM) and Clayey Sand (SC) to the maximum explored depth of 25 feet.

8.4 Groundwater Level

Groundwater was not encountered in the soil borings during our field drilling. Fluctuations of the groundwater levels on this project may be expected to occur seasonally as a result of rainfall, surface runoff, and immediate area construction activities. Elevated groundwater levels can be anticipated especially following heavy rainfall or during the wet season.

9.0 FOUNDATION RECOMMENDATIONS

The foundation system for the proposed structures must satisfy two independent engineering criteria with respect to foundation soils. First, the foundation system should be designed with an appropriate factor of safety against failure of the foundation soils. Second, the movement to the foundation system due to compression (consolidation) or expansion (swell) of the soils supporting the foundation system must be within tolerable limits for the structure.

A review of the collected subsurface data on this project, as summarized in this geotechnical report, reveals the proposed building can be supported on shallow spread footing foundation. Our foundation recommendations are presented in the following report sections.

9.1 Spread Footing Design

The building can be supported by shallow spread footing foundations bearing at 2.5 ft below the lowest adjacent finished grade. Square foundations may be sized using a net allowable bearing pressure of 2.0 kips/ft^2 for total load. Strip foundations may be sized using a net allowable bearing pressure of 1.5 kips/ft^2 for total load. These bearing pressures may be increased by 33% for transient loads such as wind.

The allowable amount of settlement that a structure may tolerate is dependent on several factors including uniformity of settlement, time rate of settlement, structural dimensions, and properties of the structural materials. Generally, the total or uniform settlement does not damage a structure but may affect drainage and utility connections. These can generally tolerate movements of one inch and more for building construction. In contrast, differential settlement affects a structure's frame and is limited by structural flexibility.

Total long-term settlement is expected to be about 1 inch. Differential settlement of the order of ½ to ¾ inch.

9.2 Spread Footing Foundation Construction

Excavations should be sloped as necessary to prevent slope failures and to allow backfilling activities. Based upon the soil types and conditions encountered during our subsurface exploration, we recommend that all excavations less than 20 feet in depth follow OSHA regulations (Standards - 29 CFR) for maximum allowable slopes. The near-surface soil profile on this project site is characterized with Type B soils. Excavations in Type B soils should be no steeper than 1-horizontal to 1-vertical. This recommended slope is for temporary slopes. Minor sloughing should be expected. Flatter slopes would improve performance. Where lateral confinement will not permit slopes to be laid back, the excavation should be shored in accordance with OSHA requirements. The shoring system, if designed, shall be stamped by the design engineer who is licensed in the State of Texas.

To reduce slope instability caused by equipment loads on top of the slopes, we recommend that concrete trucks and other heavy construction equipment should not be permitted within a distance less than the excavation depth from the top edge of the excavation. In addition, the area adjacent to the top of the excavation should not be used for temporary material storage or as a staging area for construction equipment. As provided by OSHA, the contractor designated "Competent Person," one who can evaluate the types of soil and the appropriateness of the retention system for the soil type, should monitor the excavations several times daily and as necessary to ensure worker safety. Provisions for maintaining workman safety within excavations is the sole responsibility of the contractor.

The footings should be neat excavated. Excavations should be accomplished with a smooth-mouth bucket. If a toothed bucket is used, excavation with this bucket should be stopped 12-in above final grade and the excavation completed with a smooth-mouthed bucket or by hand. If water in excess of about 2-in. accumulates at the bottom of the excavation, the water should be pumped out before concrete placement.

Steel should be placed, and the footing concreted the same day of excavation. Reinforcing steel should be properly positioned and supported to ensure the design cover around the reinforcing steel is achieved. Sides of the excavation may slough to some extent with time. Sloughed soils and other debris in the bottom of the excavation should be removed prior to steel placement. If for some reason the footings cannot be poured the same day of excavation, a thin seal slab should be placed to protect the exposed foundation soils

Significant variations can occur over short horizontal distances from the boring locations. Our representative should be present during foundation construction to verify that the proper bearing stratum has been reached, the foundation dimensions are as designed, the reinforcing steel is as specified, and that the excavation is clean and dry before reinforcing and concrete placement.

10.0 FLOOR SLAB SYSTEMS

10.1 Building Subgrade

To provide proper support for the slab on grade, we recommend one-ft thick buffer of select fill soil be prepared beneath the slab. The slab should be elevated above the surrounding grade to encourage runoff water to flow away from the slab.

10.2 Subgrade Modulus

Reinforced concrete ground floor slabs are typically designed utilizing the modulus of subgrade reaction (k). For short-term transient loads such as traffic or wheel loads, slabs, or pavements may be designed considering a subgrade reaction modulus of 100 pci. These values are based on estimated loadings, current grade levels, flexible floor slabs, and implementation of the site preparation recommendations provided herein. If higher modulus values are required, Thompson should be contacted for further evaluations.

10.3 Vapor Barrier

ACI 302.1R, Guide for Concrete Floor and Slab Construction recommends that a vapor retarder with a permeance of less than 0.3 US perms (ASTM E 96). The thickness of the vapor retarder should not be less than 10 mils, placed under the concrete floor slab on ground to reduce the transmission of water vapor from the supporting soil through the concrete slab.

10.4 Utility Bedding and Backfill

Utility trenches should be excavated in accordance with applicable OSHA requirements and observed after excavation to verify suitable bearing conditions. Any soft, loose, or unstable materials encountered at the trench bottom should be removed and replaced with approved compacted fill. Utilities should be bedded on a minimum of 4 inches of select granular material, crushed stone, or sand meeting project specifications and shaped to provide uniform support along the bottom of the pipe. The pipe embedment zone, extending from the trench bottom to at least 12 inches above the utility, should consist of approved bedding material placed in thin lifts and compacted to a minimum of 95 percent of the Standard Proctor maximum dry density (ASTM D698).

To reduce the potential for moisture migration through utility trenches beneath the proposed structure, we recommend constructing compacted clay plugs at all locations where utility lines enter and exit the building footprint. The clay plugs should consist of low-permeability clay extending the full width and depth of the trench and have a minimum length of 3 feet measured parallel to the utility alignment. Clay plug material should be compacted to at least 95 percent of the Standard Proctor maximum dry density (ASTM D698) at a moisture content ranging from 2 to 4 percentage points above optimum. The clay plugs are intended to interrupt potential seepage pathways created by utility trenches and reduce moisture migration beneath the structure.

10.5 Slab Isolation

Soil-supported slabs should be isolated from the foundations and foundation- supported elements of the structure so that differential movement between the foundations and slab will not induce excessive shear and bending stresses in the floor slab. Where the structural configuration prevents the use of a free-floating slab such as in a drop-down footing/monolithic slab configuration, the slab should be designed with suitable reinforcement and load transfer devices to preclude overstressing of the slab.

11.0 SITE DEVELOPMENT CONSIDERATIONS

The recommendations provided below are based on the project information described herein, the available subsurface data, our engineering evaluation of geotechnical matters, and our past experience. If project information or design concepts change, we should be advised of these changes in writing and provided with an opportunity to review our recommendations as presented in this geotechnical report in light of the new design information.

11.1 Wet Weather Site Conditions

The surficial soil consists of silty sands If construction is initiated in the dry weather, this soil can become dry and hard. If construction is initiated just after or during a rainy season or rain events, then the surficial soil can become wet and weak and experience pumping, unable to support construction equipment in some circumstances. There are several options that can be taken to resolve this issue:

1. Remove the wet, weak soil and replace with select fill, properly compacted, or
2. Wait until the weather dries the soil and then begin or resume construction. Tilling will help expedite the drying process only if additional rain does not occur before the soil is compacted, or
3. Use chemical additives such a lime/fly ash blend to dry the soil as well as bind the soil particles together.

If this situation of wet, weak soil due to rains and poor drainage occurs, the contractor should be prepared to use an appropriate procedure to complete construction. It is suggested that one or all of these methods be placed in the bid documents so that there will be a bid item on which to base an economic judgment of the solution to fit the goals of the owner

11.2 Drainage

We recommend positive drainage measures be established and maintained on the project site during construction. We further recommend permanent measures be implemented to maintain positive drainage on the project parcel throughout the life of the project. Runoff should be directed away from the foundation or pavement areas. Improper drainage can have significant negative effects on the performance, especially if the structure were constructed during or immediately after a dry period. The following are general notes concerning proper drainage considerations:

- Positive drainage away from the structure must be designed, constructed, and maintained throughout the structure life.
- Landscaping systems must maintain the positive drainage away from the structure and not permit water to impound adjacent to the structure.
- Downspouts from roof drainage systems and air conditioning unit drains should be designed to discharge water away from, and preferably 10-ft or more from, the foundation.
- Drainage through drainpipes to the storm sewer is preferred for all roof drains.
- Splash blocks are not effective in draining water away from the foundation and should not be used.
- Water drains should be tied to the storm sewer and not be allowed to drain along the boundary of the building with discharge at the foundation.

11.3 Trees and Landscaping

Although landscaping within the limits of the project will be limited to the perimeter, trees and other landscaping adjacent to the structure can have dynamic effects on soil moisture content. As a tree or other landscaping grows and matures, an increasing amount of moisture is needed to sustain its growth. If sufficient moisture is unavailable from infiltration of water from the surface, either through rainfall or irrigation, the moisture within the soil becomes the available source resulting in decreases in the soil moisture content and soil shrinkage adjacent to trees. The lower moisture contents generally are observed throughout the area of influence of the tree's root system. The lateral extent where moisture changes occur is generally taken as 1.5 canopy width of the tree. If trees are anticipated, they should be located a distance away from the foundation at least equal to the mature height of the tree.

11.4 Site Preparation

Locations of any existing underground utility and services lines/conduits within the project construction areas should be established. Provisions should then be made to relocate interfering utilities to appropriate locations. It should be noted that if underground pipes are not properly removed or plugged, they may serve as conduits for subsurface erosion which may subsequently lead to excessive settlement of overlying structure(s), such as ground floor slabs, concrete flatwork and pavements.

1. Clear all structure footprints and extend 5 feet beyond the perimeter. Undocumented fill soils and any buried service lines and structures should be removed in their entirety from beneath the structure footprint.

2. The bottom of the required excavated subgrade improvement zone, the entire building and pavement areas should be proof-rolled, using a fully loaded dump truck or similar, to help locate surficial and localized zones of very loose soils not encountered in the test soundings. Proof-rolling can be performed in accordance with Item 216 of TxDOT Specification. A qualified Geotechnical Engineer or his representative should be present to observe and document the proof-rolling operations. Soft or pumping soils identified during proof-rolling activities should be undercut to firm bearing soils and backfilled with select fill.

3. A qualified Soils Technician should inspect and probe the slab and footing undercut surfaces for zones of soft or very loose soils. Additional undercutting, if deemed necessary, shall be directed by the project Geotechnical Engineer of Record. Backfill materials beneath pavements, footings, and slabs shall adhere to material recommendations found in the following report section.

11.5 Fill Soil Placement

1. Select Fill shall be a locally available soil material herein defined as sandy lean clay or lean clay soil, free of organics, debris, and otherwise deleterious materials. The select fill should have at least 50% passing the No. 200 sieve and have a PI between 12 and 20, with a liquid limit less than 35. The effective diameter of clay clods should not be larger than 3 inches. Fill soils should be placed with loose lift thicknesses of not more than 6 to 8 inches. All Select Fill soils should be compacted to a minimum of 95 percent of the soil's Standard Proctor Density as per ASTM D-698. The moisture content of the select fill soils should be -1% and +3% of the optimum moisture content based on ASTM D-698.

2. Within non-saturated soils conditions at this site, compaction efforts should implement a compactor of appropriate type, size, and weight. The fill areas should be subjected to several

overlapping passes of the compaction equipment to achieve compact standards and completeness of the lift.

3. Prior to initiating fill/grading activities, representative samples of the fill soils and planned subgrade soils should be collected for classification and Proctor testing. The maximum dry density, optimum moisture content, gradation, and plasticity should be determined for each type of soil. These laboratory tests are needed for quality control during the fill and subgrade compaction.
4. Field density tests should be performed on the compacted fill soils. At a minimum, a density test should be performed for every lift of fill soils of 2,500 sq. ft. area or minimum 2 density tests per lift, whichever is greater.
5. Involvement of Thompson geotechnical engineering personnel during site work activities will help to verify that procedures and results are as specified and as anticipated. Issues identified during this process should be addressed by the Geotechnical Engineer.

12.0 PAVEMENTS

We understand that site pavement will consist of concrete or asphalt pavement. The pavement would be subject to passenger vehicles and small- to large-size delivery trucks. It is understood that the pavement design should be based on a daily traffic value of 1.54 Equivalent 18-kip Single Axle Loads (ESAL) for the standard duty sections and a daily traffic value of 4.20 ESALs for the heavy-duty sections. Our pavement recommendations for concrete and asphalt pavement are presented in the following report sections.

12.1 Pavement Subgrade Preparation

The entire subgrade should be proof rolled a maximum of 24 hours prior to paving with a loaded water truck or end dump having a single or tandem axle with 18-kip axle loads, or similar heavy rubber tired vehicle, to detect soft or loose areas. All areas exhibiting unstable subgrade conditions, such as rutting, pumping, or excessive movement should be excavated to a firm soil layer or to a maximum depth of 2 feet, whichever is shallowest, and replaced with suitable compacted fill. If unstable subgrade conditions persist, Thompson should be contacted for consultation. Soft spots should be stabilized prior to placement of pavement sections. Positive drainage away paving surfaces should be provided.

On-site soils or comparable materials may be used for any new fill. The Standard Proctor Method should be used to compact these materials to at least 95% of the Maximum Dry Density at or above optimum (ASTM D-698). Before the pavements are installed, care should be taken to confirm and maintain the appropriate moisture levels in the reworked soils.

12.2 Concrete Pavement Sections

Typical pavement sections are provided below in **Table 1**. In some cases, jurisdictional standards for pavement section construction may exceed those provided below. In that case, the pavement sections should follow the jurisdictional standards.

Table 1 – Concrete Pavement Section

Material Description	Portland Cement Concrete (PCC) Pavement	
	Standard Duty	Heavy Duty
Portland Cement Concrete	5 inches	6 inches
Aggregate Base	6 inches	6 inches

The pavement should be underlain by a minimum 6-inch-thick layer of dense-graded aggregate base course, Item 247 of the TxDOT Standard Specifications, Type A, Grade 1-2. This base layer should be compacted to at least 95 percent of the Modified Proctor Density as per ASTM D-1557 within two percentage points of the material's optimum moisture content.

Positive drainage away from the paved areas should be provided to avoid water being trapped within the aggregate base beneath the paving. Water trapped in the aggregate base can cause pumping at the concrete paving joint when subjected to traffic loading. Surface and subsurface drainage are crucial factors to take into account while designing and constructing pavements. Softening of the subgrade and other issues associated with pavement deterioration are to be expected where standing water accumulates, either on the pavement surface or in the base course layer. Additionally, adequate drainage should lessen the likelihood that the subgrade materials will become saturated over the pavement's typical service life. The following specifications should guide the design, construction, and testing of pavement:

1. Reinforcing steel may consist of #3 reinforcing steel bars placed at 18 inches in the center each way.
2. At 28 days, Portland cement concrete must have a minimum compressive strength of 4,000 pounds per square inch.

Pavement performance depends on the location and design of joints. The use of appropriate joint seals and dowels is necessary to transfer load at joints and maintain watertight joints. To regulate the placement of cracks that emerge as the concrete dries, control joints in new pavement should be sawed within 5 to 12 hours after the concrete is placed. It is recommended that design and construction procedures comply with the recommendations found in the latest edition of Design and Control of Concrete Mixtures, Portland Cement Association. Specifically, maximum crack control joint spacing of 12 and 15 feet is recommended for five (5), and six (6) inch or greater thick pavements, respectively. Joints should be thoroughly cleaned and sealed to prevent water, tiny stones, etc. from seeping in.

12.3 Asphalt Pavement Sections

Asphalt paving may be planned for this project. Our recommended design sections for asphalt paving are presented in Table 2.

Table 2 – Asphalt Pavement Section

Material Description	Asphalt Pavement	
	Standard Duty	Heavy Duty
Asphalt Surface Course	2 inches	2 inches
Asphalt Binder Course	3 inches	4 inches
Aggregate Base	6 inches	6 inches

The pavement base course should be dense-graded aggregate base course, Item 247 of the TxDOT Standard Specifications, Type A, Grade 1-2. This base layer should be compacted to at least 95 percent of the Modified Proctor Density as per ASTM D-1557 within two percentage points of the material's optimum moisture content.

During the paving life, it will likely be necessary to install a seal coat, a thin asphalt surface treatment, as regular maintenance to waterproof the surface course (reduce water penetration to paving). Additionally, it is often preferable, depending upon the conditions of the paving, to install a 1 to 1½-in. overlay (topping) near the mid-point of the design life. As with the concrete paving sections, maintenance to seal surface cracks within the asphalt section must be performed routinely to prevent degradation of the subgrade. Because of their flexibility, the performance of a flexible pavement system is highly dependent on the integrity of the underlying subgrade. Good drainage must be maintained adjacent to the pavement edges.

13.0 CONSTRUCTION RELATED GEOTECHNICAL SERVICES

To provide continuity between the geotechnical design and construction phases of the project and to assure conformance to design specifications, Thompson should be retained to provide supporting consulting and quality assurance services, during the earthwork and foundation construction phases of the project.

14.0 REPORT LIMITATIONS

This report is prepared for exclusive use by **SLI Group, Inc.** and is prepared in accordance with the Standard of Care reasonably expected of similar geotechnical engineers, providing similar services in a similar locale. No warranty is expressed or implied and all such warranties are disclaimed. This report is prepared for a limited purpose as further detailed by the objectives and/or scope work identified herein.

The evaluations and recommendations submitted in this geotechnical report are based in part upon the data obtained from the field exploration program. The nature or extent of variations throughout the subsurface profile may not become evident until the time of construction. If variations then appear evident, it may be necessary to reevaluate our recommendations as provided in this geotechnical report. Furthermore, the recovered samples were not examined, either visually or analytically, for chemical composition or environmental hazards.

The soil borings or other subsurface tests presented in this report were performed in support of the geotechnical analyses and recommendations as defined by the scope of services and not for determining the presence or extent of any subsurface debris which may exist at the site. Depending on project location, subsurface conditions, and the history of the site, buried debris, environmental contamination, or other soil types and conditions not identified may be encountered during construction. It should be noted that the project site is or may be located on a site with prior construction or other activity on the site.

We recommend Thompson be retained to review final foundation and earthwork specifications and drawings, in order to ascertain whether our recommendations contained in this report have been properly interpreted and implemented.

This report has not been prepared as, and should not be used as, a design or specification document to be directly implemented by the Contractor. In addition, this report was not prepared for the purposes of bid development, and the accuracy of this report is limited. If included as part of a bid package, it is for informational purposes only, and it shall remain the Contractor's responsibility to retain and confer with a geotechnical engineer to obtain specific types of information the Contractor or others may need or prefer to interpret this report or perform additional geotechnical testing prior to bidding and construction.

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Appendix A

Test Location Plan and Records of Test Borings

	PREPARED BY:	PROJECT	LOCATION:
thompson ENGINEERING TBPELS Firm Registration No. F-4824	B.Sebastiao **Title:** Boring Location Plan **Date:** August 10, 2026	**Name:** National Bank & Trust **Number:** 26-1119-0030 **Client:** SLI Group, Inc.	Sam Houston Drive and FM 969 Bastrop, Texas

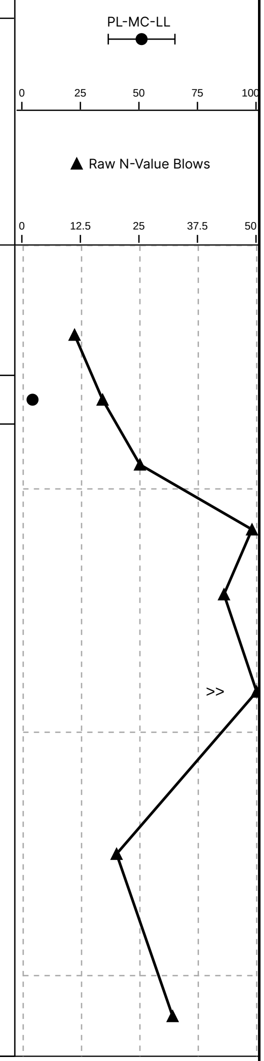
Nation Bank & Trust

Sam Houston Drive and FM 969 Bastrop, Texas

B-1

Drilling Co.:	AF Drilling	Project No.:	26-1119-0030	Remarks: -
Driller:	Carlos	Date Drilled:	07/22/2026	
Logged By:	Andres	Boring Depth:	25.00'	
Equipment:	simco 2400	Boring Elevation:	413.00'	
Coordinates:	30.123613, -97.366121	Client Name:	SLI Group, Inc.	

Depth (ft)	Elevation (ft)	Graphic Log	Visual Classification and Remarks	Samples: Sample Graphic	Samples: Blow Counts (N-Value)	Lab: Moisture Content (%)	Lab: % Fines
0	413	SM	**SILTY SAND (SM)**; medium dense, brown, moist	Auger			
	410			SPT	7-6-5 (11)		
5		SP-SM	**POORLY GRADED SAND WITH SILT (SP-SM)**; medium dense to very dense, brown, moist	SPT	8-9-8 (17)	4.0	10.0
	405			SPT	10-12-13 (25)		
			- Gravel Below 8 Feet	SPT	15-25-24 (49)		
10				SPT	13-20-23 (43)		
	400			SPT	15-20-33 (53)		
15							
	395			SPT	12-9-11 (20)		
20							
	390	SC	**CLAYEY SAND (SC)**; dense, brown, moist to wet with Gravel	SPT	7-12-20 (32)		
25			Boring Terminated @ 25 feet				

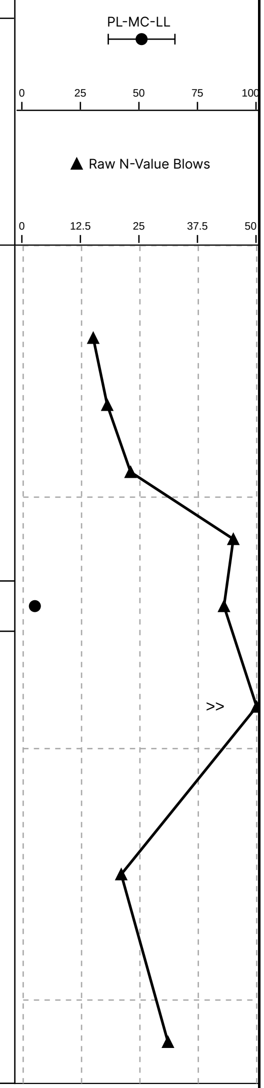


SM	SC	SPT - Standard Penetration	-
SP-SM	Auger - Auger Sample	-	-

thompson ENGINEERING	**Nation Bank & Trust** **Sam Houston Drive and FM 969 Bastrop, Texas**	**B-2** Page 1 of 1

Drilling Co.:	AF Drilling	Project No.:	26-1119-0030	Remarks: -
Driller:	Carlos	Date Drilled:	07/22/2026	
Logged By:	Andres	Boring Depth:	25.00'	
Equipment:	simco 2400	Boring Elevation:	413.00'	
Coordinates:	30.123509, -97.365997	Client Name:	SLI Group, Inc.	

Depth (ft)	Elevation (ft)	Graphic Log	Visual Classification and Remarks	Samples: Sample Graphic	Samples: Blow Counts (N-Value)	Lab: Moisture Content (%)	Lab: % Fines
0	413	SM	**SILTY SAND** (**SM**); medium dense to very dense, brown, moist silty	Auger			
	410	SM		SPT	6-8-7 (15)		
5		SM		SPT	7-9-9 (18)		
	405	SM		SPT	10-11-12 (23)		
		SM	- Gravel Below 8 Feet	SPT	13-19-26 (45)		
10		SM		SPT	16-18-25 (43)	5.0	16.0
	400	SM		SPT	19-21-30 (51)		
15	395	SM					
		SC	**CLAYEY SAND** (**SC**); dense, brown, moist to wet with Gravel	SPT	10-10-11 (21)		
20	390	SC					
		SC		SPT	10-12-19 (31)		
25			Boring Terminated @ 25 feet				



SM	SC	Auger - Auger Sample	SPT - Standard Penetration

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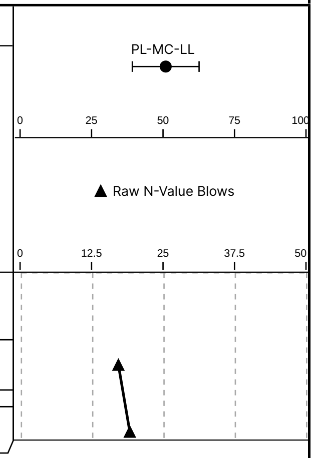
Drilling Co.:	AF Drilling	Project No.:	26-1119-0030	Remarks: -
Driller:	Carlos	Date Drilled:	07/22/2026	
Logged By:	Andres	Boring Depth:	5.00'	
Equipment:	simco 2400	Boring Elevation:	413.00'	
Coordinates:	30.123813, -97.365855	Client Name:	SLI Group, Inc.	

Depth (ft)	Elevation (ft)	Graphic Log	Visual Classification and Remarks	Samples: Sample Graphic	Samples: Blow Counts (N-Value)
	413				
			SILTY SAND (**SM**); medium dense, brown, moist silty	Auger	
	410			SPT	7-8-9 (17)
5				SPT	7-9-10 (19)



Boring Terminated @ 5 feet

SM | Auger - Auger Sample | SPT - Standard Penetration Test | -

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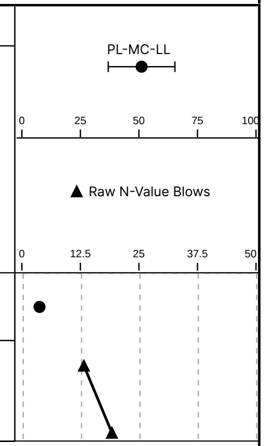
Drilling Co.:	AF Drilling	Project No.:	26-1119-0030
Driller:	Carlos	Date Drilled:	07/22/2026
Logged By:	Andres	Boring Depth:	5.00'
Equipment:	simco 2400	Boring Elevation:	414.00'
Coordinates:	30.123269, -97.366192	Client Name:	SLI Group, Inc.

Remarks:
-

Depth (ft)	Elevation (ft)	Graphic Log	Visual Classification and Remarks	Samples: Sample Graphic	Samples: Blow Counts (N-Value)	Lab: Moisture Content (%)	Lab: % Fines
	414		**SILTY SAND** (**SM**); medium dense, brown, moist silty	Auger		7.0	13.0
	410			SPT	8-7-6 (13)		
5				SPT	10-9-10 (19)		
			Boring Terminated @ 5 feet				



SM | Auger - Auger Sample | SPT - Standard Penetration Test | -

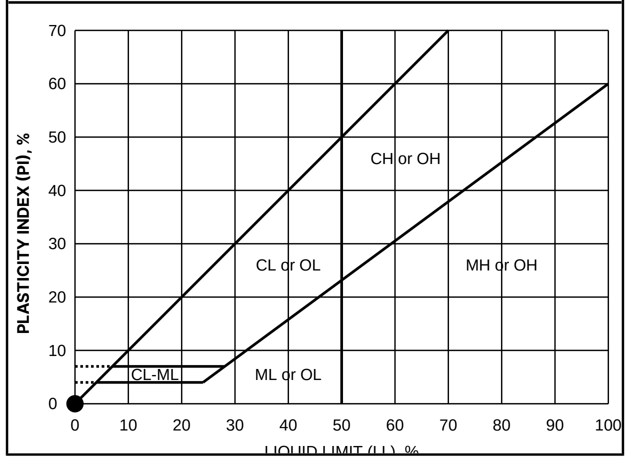
SOIL CLASSIFICATION CHART PER ASTM D 2488

PRIMARY DIVISIONS			SECONDARY DIVISIONS		
			GROUP SYMBOL		GROUP NAME
COARSE-GRAINED SOILS more than 50% retained on No. 200 sieve	GRAVEL more than 50% of coarse fraction retained on No. 4 sieve	CLEAN GRAVEL less than 5% fines		GW	well-graded GRAVEL
				GP	poorly-graded GRAVEL
		GRAVEL with DUAL CLASSIFICATIONS 5% to 12% fines		GW-GM	well-graded GRAVEL with silt
				GP-GM	poorly-graded GRAVEL with silt
				GW-GC	well-graded GRAVEL with clay
				GP-GC	poorly-graded GRAVEL with clay
		GRAVEL with FINES more than 12% fines		GM	silty GRAVEL
				GC	clayey GRAVEL
				GC-GM	silty, clayey GRAVEL
	SAND 50% or more of coarse fraction retained on No. 4 sieve	CLEAN SAND less than 5% fines		SW	well-graded SAND
				SP	poorly-graded SAND
		SAND with DUAL CLASSIFICATIONS 5% to 12% fines		SW-SM	well-graded SAND with silt
				SP-SM	poorly-graded SAND with silt
				SW-SC	well-graded SAND with clay
				SP-SC	poorly-graded SAND with clay
		SAND with FINES more than 12% fines		SM	silty SAND
				SC	clayey SAND
				SC-SM	silty, clayey SAND
FINE-GRAINED SOILS 50% or more passes No. 200 sieve	SILT and CLAY liquid limit less than 50%	INORGANIC		CL	lean CLAY
				ML	SILT
				CL-ML	silty CLAY
		ORGANIC		OL (PI > 4)	organic CLAY
				OL (PI < 4)	organic CLAY
	SILT and CLAY liquid limit 50% or more	INORGANIC		CH	fat CLAY
				MH	elastic SILT
		ORGANIC		OH (plots on or above 'A'-line)	organic CLAY
				OH (plots below 'A'-line)	organic SILT
	Highly Organic Soils			PT	Peat

GRAIN SIZE

DESCRIPTION		SIEVE SIZE	GRAIN SIZE	APPROXIMATE SIZE
Boulders		> 12"	> 12"	Larger than basketball-sized
Cobbles		3 - 12"	3 - 12"	Fist-sized to basketball-sized
Gravel	Coarse	3/4 - 3"	3/4 - 3"	Thumb-sized to fist-sized
	Fine	#4 - 3/4"	0.19 - 0.75"	Pea-sized to thumb-sized
Sand	Coarse	#10 - #4	0.079 - 0.19"	Rock-salt-sized to pea-sized
	Medium	#40 - #10	0.017 - 0.079"	Sugar-sized to rock-salt-sized
	Fine	#200 - #40	0.0029 - 0.017"	Flour-sized to sugar-sized
Fines		Passing #200	< 0.0029"	Flour-sized and smaller

PLASTICITY CHART



APPARENT DENSITY - COARSE-GRAINED SOIL

APPARENT DENSITY	SPOOLING CABLE OR CATHEAD		AUTOMATIC TRIP HAMMER	
	SPT (blows/foot)	MODIFIED SPLIT BARREL (blows/foot)	SPT (blows/foot)	MODIFIED SPLIT BARREL (blows/foot)
Very Loose	≤ 4	≤ 8	≤ 3	≤ 5
Loose	5 - 10	9 - 21	4 - 7	6 - 14
Medium Dense	11 - 30	22 - 63	8 - 20	15 - 42
Dense	31 - 50	64 - 105	21 - 33	43 - 70
Very Dense	> 50	> 105	> 33	> 70

CONSISTENCY - FINE-GRAINED SOIL

CONSISTENCY	SPOOLING CABLE OR CATHEAD		AUTOMATIC TRIP HAMMER	
	SPT (blows/foot)	MODIFIED SPLIT BARREL (blows/foot)	SPT (blows/foot)	MODIFIED SPLIT BARREL (blows/foot)
Very Soft	< 2	< 3	< 1	< 2
Soft	2 - 4	3 - 5	1 - 3	2 - 3
Firm	5 - 8	6 - 10	4 - 5	4 - 6
Stiff	9 - 15	11 - 20	6 - 10	7 - 13
Very Stiff	16 - 30	21 - 39	11 - 20	14 - 26
Hard	> 30	> 39	> 20	> 26